

The Boundary Element Method in Elastostatics

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Reciprocal theorem

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Boundary Integral Equation

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Overview

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- Boundary integral equation
- Boundary element method
- Re-analysis and interactivity
- Enrichment of approximation space



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Introduction – a cursory overview



FEM

More versatile Domain method



BEM

Computationally more demanding Simple meshing Solution accuracy



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Infinite domain problems



Elements are only used on the two internal boundaries in this wave problem



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Introduction – prelude to the theoretical development

For the purposes of this lecture we will start by stating the reciprocal theorem from a structural mechanics viewpoint.

The notes contain a fuller description with greater mathematical rigour. The reciprocal theorem is developed from a weighted residual expression.

We will confine ourselves to the collocation BEM. There is a popular Galerkin form of BEM too.



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Reciprocal theorem





Real load case:

Tractions: *t*_i Displacements: *u*_i Body forces: *b*_i Complementary load case: Tractions: t_i^* Displacements: u_i^* Body forces: b_i^*



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Reciprocal theorem

Form statements of work done by force x displacement force x displacement

$$\int_{\Gamma} \mathbf{t}_{i}^{*} \mathbf{u}_{i} d\Gamma + \int_{\Omega} \mathbf{b}_{i}^{*} \mathbf{u}_{i} d\Omega = \int_{\Gamma} \mathbf{u}_{i}^{*} \mathbf{t}_{i} d\Gamma + \int_{\Omega} \mathbf{u}_{i}^{*} \mathbf{b}_{i} d\Omega$$

Integrate surface tractions over the surface Γ

Integrate body forces over the volume $\boldsymbol{\Omega}$

This is the *reciprocal theorem* due to Maxwell and Betti.

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Reciprocal theorem statement:

$$\int_{\Gamma} t_i^* u_i d\Gamma + \int_{\Omega} b_i^* u_i d\Omega = \int_{\Gamma} u_i^* t_i d\Gamma + \int_{\Omega} u_i^* b_i d\Omega$$

In order to reduce to boundary-only, we need to eliminate the two volume integrals.

- We will simplify the development for this lecture by stipulating no body forces in the real load case, i.e. $b_i = 0$
- We will address the volume integral on the LHS by appropriate choice of a complementary load case



Fundamental solutions

Complementary load case *: Dirac delta function point load in one of the coordinate directions at some point ξ



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Fundamental solutions

 $\int^{b} g(x)\Delta(x-\xi) \, dx = g(\xi), \qquad a < \xi < b$

Properties of Dirac delta function:

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Fundamental solutions

So the displacement field in the complementary load case, u^* , is the solution to the equilibrium equation:

$$\sigma_{ij,j} + \Delta \left(x - \xi \right) e_i(x) = 0$$

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So the displacement field in the complementary load case, u^* , is the solution to the equilibrium equation:

$$\sigma_{ij,j} + \Delta \left(x - \xi \right) e_i(x) = 0$$

It turns out that the solution is:

(

 $u_i^* = U_{ik}e_k$

where

$$U_{ik} = \frac{1}{8\pi\mu (1-\nu)} \left[(3-4\nu) \ln\left(\frac{1}{r}\right) \delta_{ik} + r_{,i}r_{,k} \right]$$
(2D)

$$U_{ik} = \frac{1}{16\pi\mu \left(1 - \nu\right)r} \left[(3 - 4\nu)\delta_{ik} + r_{,i}r_{,k} \right]$$
(3D)

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Fundamental soluti

So the displacement field u^* , is the solution to the e

$$\sigma_{ij,j} + \Delta$$

It turns out that the solution

 $u_i^* = U_{ik}e_k$

where

$$U_{ik} = \frac{1}{8\pi\mu \left(1-\nu\right)} \left[\left(3-4\nu\right) \ln\left(\frac{1}{r}\right) \delta_{ik} + r_{,i}r_{,k} \right]$$
(2D)

$$U_{ik} = \frac{1}{16\pi\mu \left(1 - \nu\right)r} \left[(3 - 4\nu)\delta_{ik} + r_{,i}r_{,k} \right]$$
(3D)





Fundamental solutions

Displacement fundamental solutions:

$$U_{ik} = \frac{1}{8\pi\mu (1-\nu)} \left[(3-4\nu) \ln\left(\frac{1}{r}\right) \delta_{ik} + r_{,i}r_{,k} \right]$$
(2D)

$$U_{ik} = \frac{1}{16\pi\mu \left(1 - \nu\right)r} \left[(3 - 4\nu)\delta_{ik} + r_{,i}r_{,k} \right]$$
(3D)

Differentiate and apply Hooke's Law to arrive at the corresponding traction fundamental solutions:

$$T_{ik} = \frac{-1}{4\pi (1-\nu) r} r_{,n} \left[(1-2\nu) \delta_{ik} + 2r_{,i}r_{,k} \right] + \frac{1-2\nu}{4\pi (1-\nu) r} \left(r_{,k}n_i - r_{,i}n_k \right)$$
(2D)

$$T_{ik} = \frac{-1}{8\pi (1-\nu) r^2} r_{,n} \left[(1-2\nu) \delta_{ik} + 3r_{,i} r_{,k} \right] + \frac{1-2\nu}{8\pi (1-\nu) r^2} \left(r_{,k} n_i - r_{,i} n_k \right)$$
(3D)

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Physical significance:

Fundamental solutions provide the displacement and traction fields, in an infinite material, due to a Dirac point force at ξ .



These solutions are due to Kelvin.

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Use of the Dirac delta function as the complementary load case has given us a set of fundamental solutions from which we can easily find u^* and t^* .

The choice of the Dirac delta function also removes the remaining volume integral in the reciprocal theorem statement:

$$\int_{\Gamma} t_i^* u_i d\Gamma + \int_{\Omega} b_i^* u_i d\Omega = \int_{\Gamma} u_i^* t_i d\Gamma + \int_{\Omega} u_i^* b_i d\Omega$$
$$\int_{\Omega} b_i^* u_i d\Omega = \int_{\Omega} \Delta \left(x - \xi \right) e_i u_i d\Omega = u_i \left(\xi \right) e_i$$



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Treatment of the two volume integrals leaves simply:

$$u_{k}\left(\xi\right) + \int_{\Gamma} T_{ik} u_{i} d\Gamma = \int_{\Gamma} U_{ik} t_{i} d\Gamma \qquad (*)$$

The last step in making this a boundary-only expression is to *move* ξ *to the boundary*, i.e. $\xi \in \Gamma$.

This causes complications because, when $\xi \in \Gamma$, *r* passes through zero on the boundary causing both boundary integrals to *contain singular functions*.



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The integrals containing U_{ik} are only weakly singular. The integrals in 2D, for example, have a logarithmic singularity and can be quickly evaluated using the logarithmic form of Gauss-Legendre quadrature.

$$\int_{-1}^{1} \ln\left(\frac{1}{x}\right) f(x) \, dx \simeq \sum_{i=1}^{N} f(x_i) \, w_i$$

There are other schemes – mostly involving coordinate transformation – for evaluating weakly singular integrals.



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 Γ_{ϵ}^{+}

 $\Gamma - \Gamma_{z}$

The strongly singular integrals containing T_{ik} may be taken in the *Cauchy principal value sense* (limit as radius $\rho \rightarrow 0$) causing the introduction of a multiplier c_{ik} .

$$c_{ik}\left(\xi\right)u_{k}\left(\xi\right) + \int_{\Gamma}T_{ik}u_{i}d\Gamma = \int_{\Gamma}U_{ik}t_{i}d\Gamma$$





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These integrals may be split into three parts:





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For convenience we write the last term as:

$$u_{i}(\xi)\lim_{\rho\to 0}\left\{\int_{\Gamma_{\varepsilon}^{+}}T_{ik}d\Gamma\right\} = \alpha_{ik}\left(\xi\right)u_{i}\left(\xi\right)$$

So $c_{ik} = \delta_{ik} + \alpha_{ik}$

Also define θ coordinate:





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On Γ_{ε}^{+} : $r = \rho \cos \theta e_{1} + \rho \sin \theta e_{2}$ $d\Gamma_{\varepsilon}^{+} = \rho d\theta$ $r_{,n} = 1$ $r_{,1} = \cos \theta$ $r_{,2} = \sin \theta$ $n_{1} = \cos \theta$ $n_{2} = \sin \theta$

Boundary integral equation



Substituting these into the various T_{ik} terms allows the integrals to be calculated analytically, yielding α_{ik} and then c_{ik}



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Worked example: α_{11} on a smooth boundary

Substituting the functions of *r* and *n* into T_{11} gives

$$\alpha_{11}\left(\xi\right) = \int_{\Gamma_{\varepsilon}^{+}} \left[\frac{-1}{4\pi\left(1-\nu\right)\rho}\left(1-2\nu+2\cos^{2}\theta\right)\right] d\Gamma_{\varepsilon}^{+}$$
$$\alpha_{11}\left(\xi\right) = \frac{-1}{4\pi\left(1-\nu\right)}\int_{0}^{\theta_{1}+\pi}\left(1-2\nu+2\cos^{2}\theta\right) d\theta$$





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Worked example: α_{11} on a smooth boundary

$$\alpha_{11}\left(\xi\right) = \frac{-1}{4\pi\left(1-\nu\right)} \left[\left(1-2\nu\right)\theta + \left(\theta+\cos\theta\sin\theta\right)\right]_{\theta_{1}}^{\theta_{1}+\pi}$$

$$\alpha_{11}(\xi) = \frac{-1}{4\pi (1-\nu)} \left\{ (2-2\nu) \pi + \cos \left(\theta_1 + \pi\right) \sin \left(\theta_1 + \pi\right) - \cos \theta_1 \sin \theta_1 \right\}$$

$$\alpha_{11}(\xi) = \frac{-1}{4\pi (1-\nu)} \left\{ (2-2\nu) \pi + \sin \theta_1 \cos \theta_1 - \cos \theta_1 \sin \theta_1 \right\}$$

$$\alpha_{11}\left(\xi\right) = -\frac{1}{2}$$

$$c_{11} = \delta_{11} + \alpha_{11} = 0.5$$



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Boundary integral equation

Generally the 'free term' or 'jump term' $c_{ik}(\xi)$ is determined by the angle β subtended by the material at ξ .



But.... much easier to use the row-sum method



Boundary integral equation

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$$c_{ik}\left(\xi\right)u_{k}\left(\xi\right) + \int_{\Gamma}T_{ik}u_{i}d\Gamma = \int_{\Gamma}U_{ik}t_{i}d\Gamma$$

Analytical solution is possible only for the very simplest problems. We will proceed by discretisation, leading to the **Boundary Element Method** itself.



Boundary element method

We discretise the boundary into elements.

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This provides:

- Nodal points and local interpolation to define an approximate solution, exactly like finite elements
- Convenient small portions of the boundary to perform numerical integration accurately
- A set of node points on which to place ξ in turn to provide a square system of linear equations



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Boundary elements share many essential characteristics with finite elements:

η = 0 $\eta = 1$ $\eta = -1$ $N_1(\eta) = \frac{\eta}{2}(\eta - 1)$ $N_2(\eta) = (1 - \eta)(1 + \eta)$ $N_3(\eta) = \frac{\eta}{2}(\eta + 1)$ $u_{i}(\eta) = \sum_{j=1}^{\circ} N_{j}(\eta) u_{i}^{jm}$



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Discrete form of the boundary integral equation:

$$c_{ik}\left(\xi\right)u_{k}\left(\xi\right) + \sum_{m=1}^{M}\int_{\Gamma_{m}}T_{ik}u_{i}d\Gamma = \sum_{m=1}^{M}\int_{\Gamma_{m}}U_{ik}t_{i}d\Gamma$$

Express *u* and *t* in their interpolated forms over element *m* $c_{ik}(\xi) u_k(\xi) + \sum_{m=1}^{M} \int_{\Gamma_m} T_{ik} N_p u_i^{pm} d\Gamma = \sum_{m=1}^{M} \int_{\Gamma_m} U_{ik} N_p t_i^{pm} d\Gamma$

Remove vectors of *nodal* displacements and tractions from the integrals:

$$c_{ik}\left(\xi\right)u_{k}\left(\xi\right) + \sum_{m=1}^{M}\int_{\Gamma_{m}}T_{ik}N_{p}d\Gamma \ u_{i}^{pm} = \sum_{m=1}^{M}\int_{\Gamma_{m}}U_{ik}N_{p}d\Gamma \ t_{i}^{pm}$$



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$$c_{ik}\left(\xi\right)u_{k}\left(\xi\right) + \sum_{m=1}^{M}\int_{\Gamma_{m}}T_{ik}N_{p}d\Gamma \ u_{i}^{pm} = \sum_{m=1}^{M}\int_{\Gamma_{m}}U_{ik}N_{p}d\Gamma \ t_{i}^{pm}$$

Transform boundary integrals to local coordinate system:

$$c_{ik}\left(\xi\right)u_{k}\left(\xi\right) + \sum_{m=1}^{M} \int_{-1}^{1} T_{ik}N_{p}J\left(\eta\right)d\eta \ u_{i}^{pm} = \sum_{m=1}^{M} \int_{-1}^{1} U_{ik}N_{p}J\left(\eta\right)d\eta \ t_{i}^{pm}$$

Evaluate this equation for ξ at, say, node 1 and Dirac force in *x*-direction:

$$c_{11}(1)u_1^1 + \hat{h}_{1,1}u_1^1 + \hat{h}_{1,2}u_2^1 + \hat{h}_{1,3}u_1^2 + \hat{h}_{1,4}u_2^2 + \hat{h}_{1,5}u_1^3 + \dots$$

= $g_{1,1}t_1^1 + g_{1,2}t_2^1 + g_{1,3}t_1^2 + g_{1,4}t_2^2 + g_{1,5}t_1^3 + \dots$



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$$c_{11}(1)u_1^1 + \hat{h}_{1,1}u_1^1 + \hat{h}_{1,2}u_2^1 + \hat{h}_{1,3}u_1^2 + \hat{h}_{1,4}u_2^2 + \hat{h}_{1,5}u_1^3 + \dots$$

= $g_{1,1}t_1^1 + g_{1,2}t_2^1 + g_{1,3}t_1^2 + g_{1,4}t_2^2 + g_{1,5}t_1^3 + \dots$

Embed the free term *c* into the others....

$$h_{1,1}u_1^1 + h_{1,2}u_2^1 + h_{1,3}u_1^2 + h_{1,4}u_2^2 + h_{1,5}u_1^3 + \dots$$

= $g_{1,1}t_1^1 + g_{1,2}t_2^1 + g_{1,3}t_1^2 + g_{1,4}t_2^2 + g_{1,5}t_1^3 + \dots$

... by defining for notational simplicity

 $h_{i,j} = \hat{h}_{i,j} + \delta_{ij} c_{ij} \left(\xi\right)$



Boundary element method

Evaluating the h and g terms for ξ at each node in turn gives

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Hu = Gt





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For each row, prescribe either the displacement or traction as a boundary condition, and column-swap to bring all remaining unknowns to LHS...





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The whole right hand side is now known and can be multiplied out leaving

 $A\underline{x} = \underline{b}$

This can be solved either directly or iteratively.

Choice of solver is limited by asymmetry of *A*.



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Now move ξ off the boundary and into the material. We can solve for the displacements at this internal point using the equation (*) we developed part way through the derivation

$$u_{k}\left(\xi\right) + \int_{\Gamma} T_{ik} u_{i} d\Gamma = \int_{\Gamma} U_{ik} t_{i} d\Gamma$$

Stress components at the internal point can be found from a derivative of this equation.



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Re-analysis and interactivity

One aspect of the BEM we are pursuing in Durham is reanalysis leading to an interactivity to design analysis.





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Re-analysis and interactivity

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One aspect of the BEM we are pursuing here in Durham is re-analysis leading to an interactivity to design analysis.

Make a design change.... ...only a few nodes move...

...and some rows and columns change, but most of the matrix is the same as the last one.



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Surface fits for rapid approximation of integrals

$$U_{001} = \left[2.166 \left(1 + \cos \left(2\theta \right) \right) - 8.996 \ln \left(\frac{r_m}{L} \right) \right] \times 10^{-7}$$





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Iterative re-solution

We precondition a GMRES scheme with an approximate complete LU decomposition.

First analysis: LU-decomposition (save L and U) *Re-analysis*: Use the full LU-decomposition of the original system as a preconditioner for iterative solution of the perturbed system.

	Perturbation Type				
Preconditioner	Move Point	Move circle	External Fillet Resize	Internal Fillet Resize 1	Internal Fillet Resize 2
None	30 – 50	31 – 49	36 – 48	34 – 50	37 – 53
Diagonal	39 – 53	36 – 47	36 – 44	43 – 47	44 – 50
Full LU	2 – 17	2 – 9	3 – 9	3 – 13	3 – 11
Number of iterations to convergence					



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Enriched approximation space

The shape functions form a Partition of Unity

 $\sum_{j=1}^{3} N_{j}(\xi) = 1$

We use this property to enrich using arbitrary functions

$$\sum_{j=1}^{3} N_j(\xi) \psi(\xi) = \psi(\xi)$$

If we know functions ψ that populate the particular problem solution space we can include them in our approximation and obtain improved results Melenk & Babuška.



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Enriched dual BEM for fracture mechanics

We can base an enrichment on the first order terms of the Williams expansion for displacement components around a crack tip

$$u_{j}^{n}(\xi) = \sum_{a=1}^{M} N_{a}(\xi) u_{j}^{na} + \sum_{a=1}^{M} \sum_{l=1}^{L} N_{a}(\xi) \psi_{l}^{U}(\xi) A_{jl}^{na}$$
$$\psi^{U}(\rho, \theta) = \left\{ \sqrt{\rho} \cos\left(\frac{\theta}{2}\right), \sqrt{\rho} \sin\left(\frac{\theta}{2}\right), \sqrt{\rho} \sin\left(\frac{\theta}{2}\right), \sqrt{\rho} \sin\left(\frac{\theta}{2}\right) \sin(\theta), \sqrt{\rho} \cos\left(\frac{\theta}{2}\right) \sin(\theta) \right\}^{\mathrm{T}}$$



Remark: this is the same approximation space as used in the XFEM (Moës, Dolbow, Belytschko) but in a BEM sense.



Results – mode I problem

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PU-BEM enriched basis for wave problems

The PUM multiple plane wave expansion for potential on an element





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Conclusions

- BEM has been presented for elastostatics problems
- Body forces and non-linearity can be handled by further treatment not discussed here
- Attractive for various classes of problem
- Re-analysis leads to interactivity in stress analysis
- Enrichment of the approximation space can yield improved accuracy